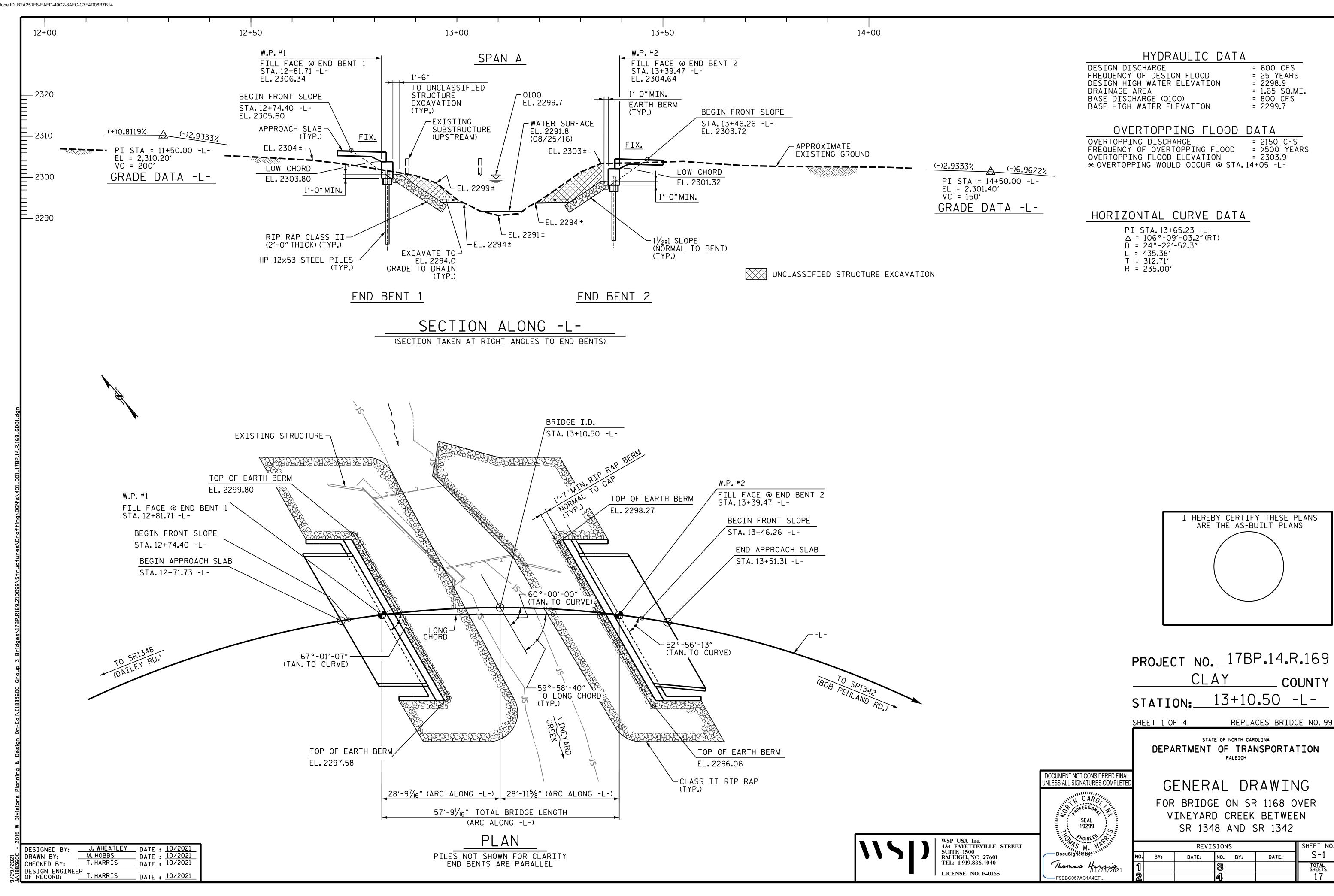
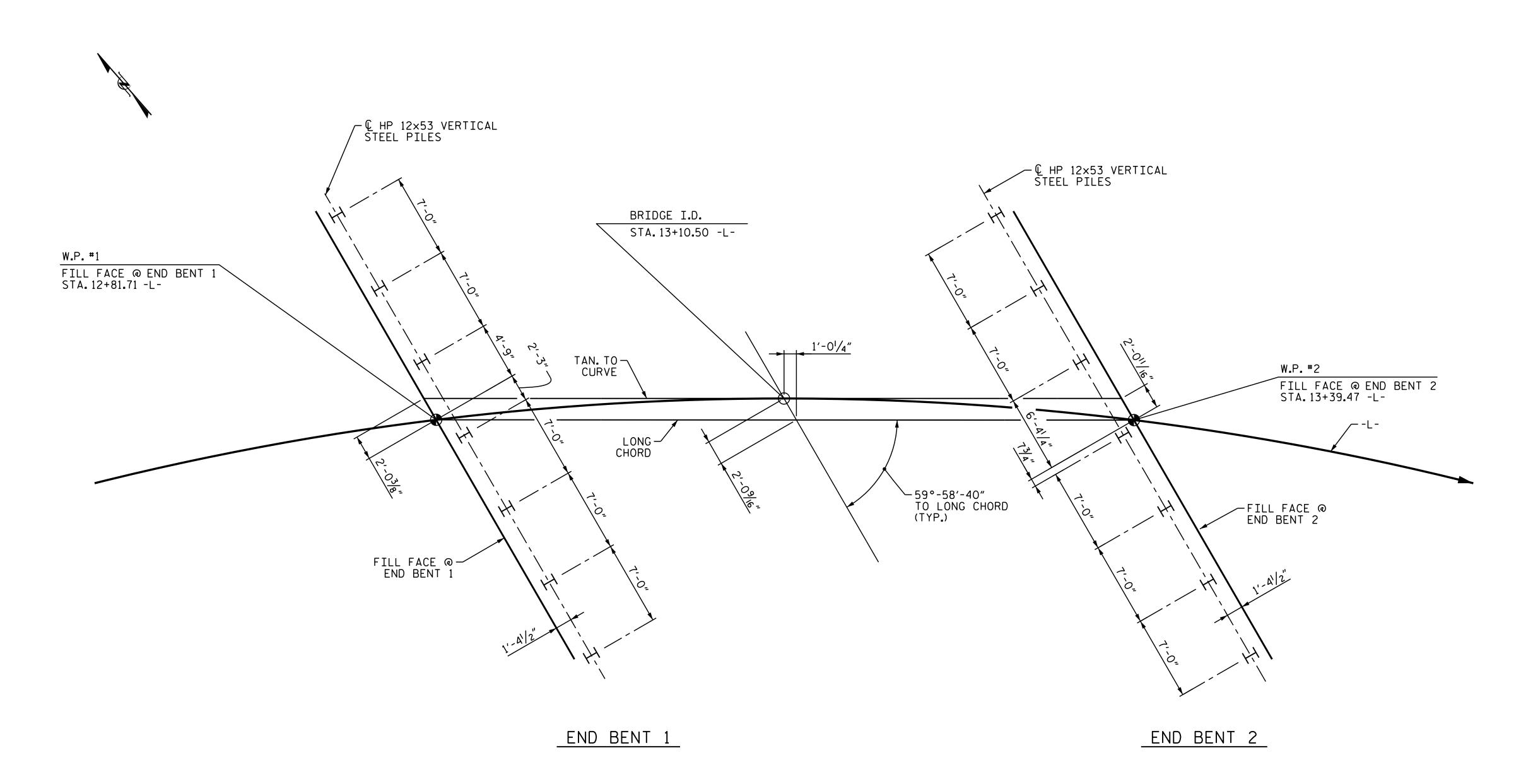
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This file or an individual page shall not be considered a certified document.





FOUNDATION LAYOUT

NOTES

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT 1 AND 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 75 TONS PER PILE.

DRIVE PILES AT END BENT 1 AND 2 TO A REQUIRED DRIVING RESISTANCE OF 125 TONS PER PILE.

DRILLED-IN PILES ARE REQUIRED TO INSTALL PILES AT END BENT 1 AND 2. EXCAVATE HOLES AT PILE LOCATIONS TO ELEVATION 2287FT. FOR PILE EXCAVATION, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILE RESTRIKES ARE NOT NECESSARY FOR PILES AT BOTH END BENTS.

PROJECT NO. 17BP.14.R.169

CLAY COUNTY

STATION: 13+10.50 -L-

SHEET 2 OF 4

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

Thomas Harris 111/23/2021

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

GENERAL DRAWING

FOR BRIDGE ON SR 1168 OVER VINEYARD CREEK BETWEEN SR 1348 AND SR 1342

 REVISIONS
 SHEET NO.

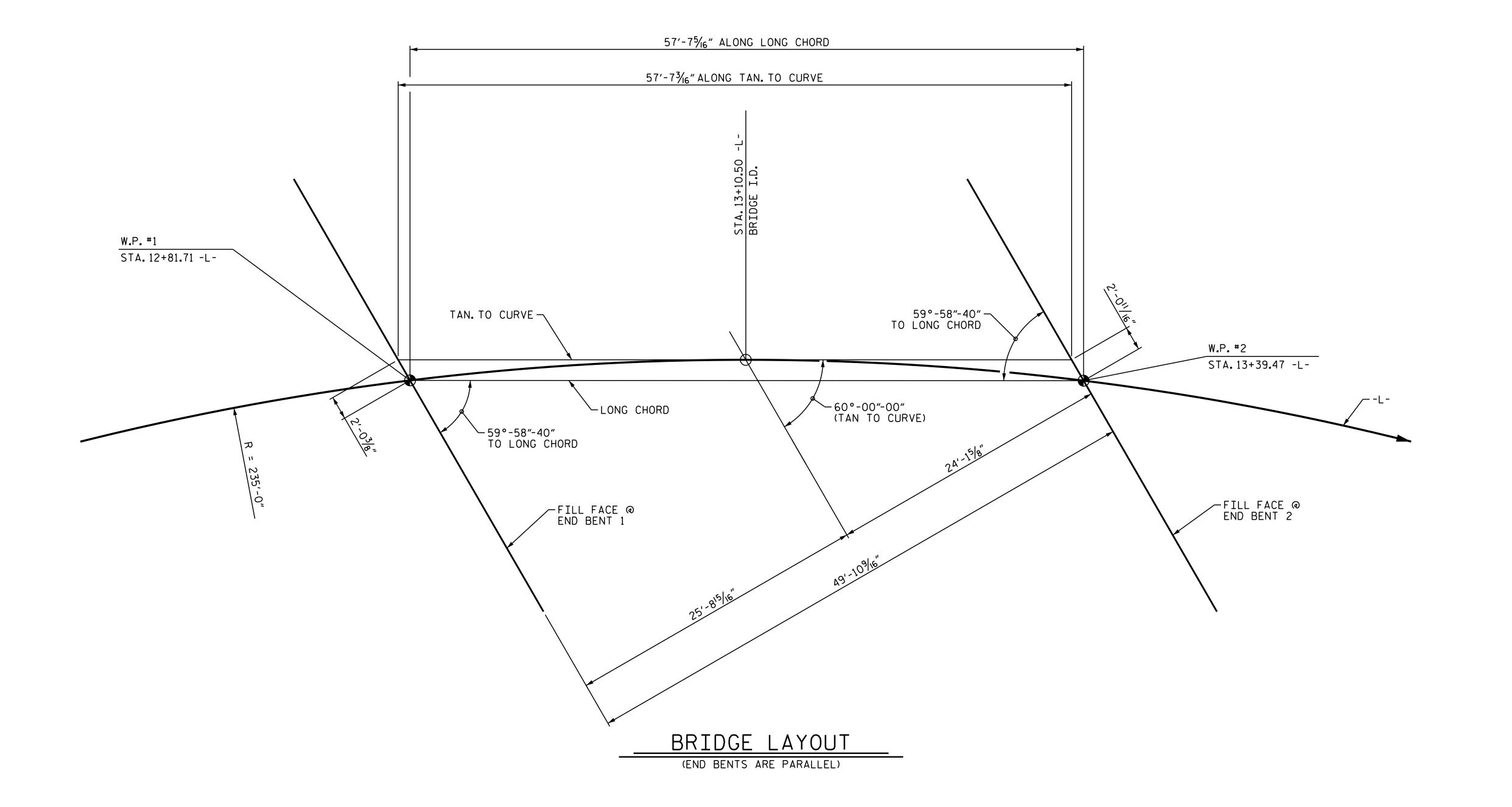
 NO.
 BY:
 DATE:
 S-2

 1
 3
 TOTAL SHEETS

 2
 4
 17

WSP USA Inc.
434 FAYETTEVILLE STREET
SUITE 1500
RALEIGH, NC 27601
TEL: 1.919.836.4040
LICENSE NO. F-0165

DESIGNED BY: J. WHEATLEY DATE: 10/2021
DRAWN BY: M. HOBBS DATE: 10/2021
CHECKED BY: T. HARRIS DATE: 10/2021
DESIGN ENGINEER
OF RECORD: T. HARRIS DATE: 10/2021



PROJECT NO. 17BP.14.R.169

CLAY COUNTY

STATION: 13+10.50 -L-

SHEET 3 OF 4

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

Thomas Harris 111/23/2021

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

GENERAL DRAWING

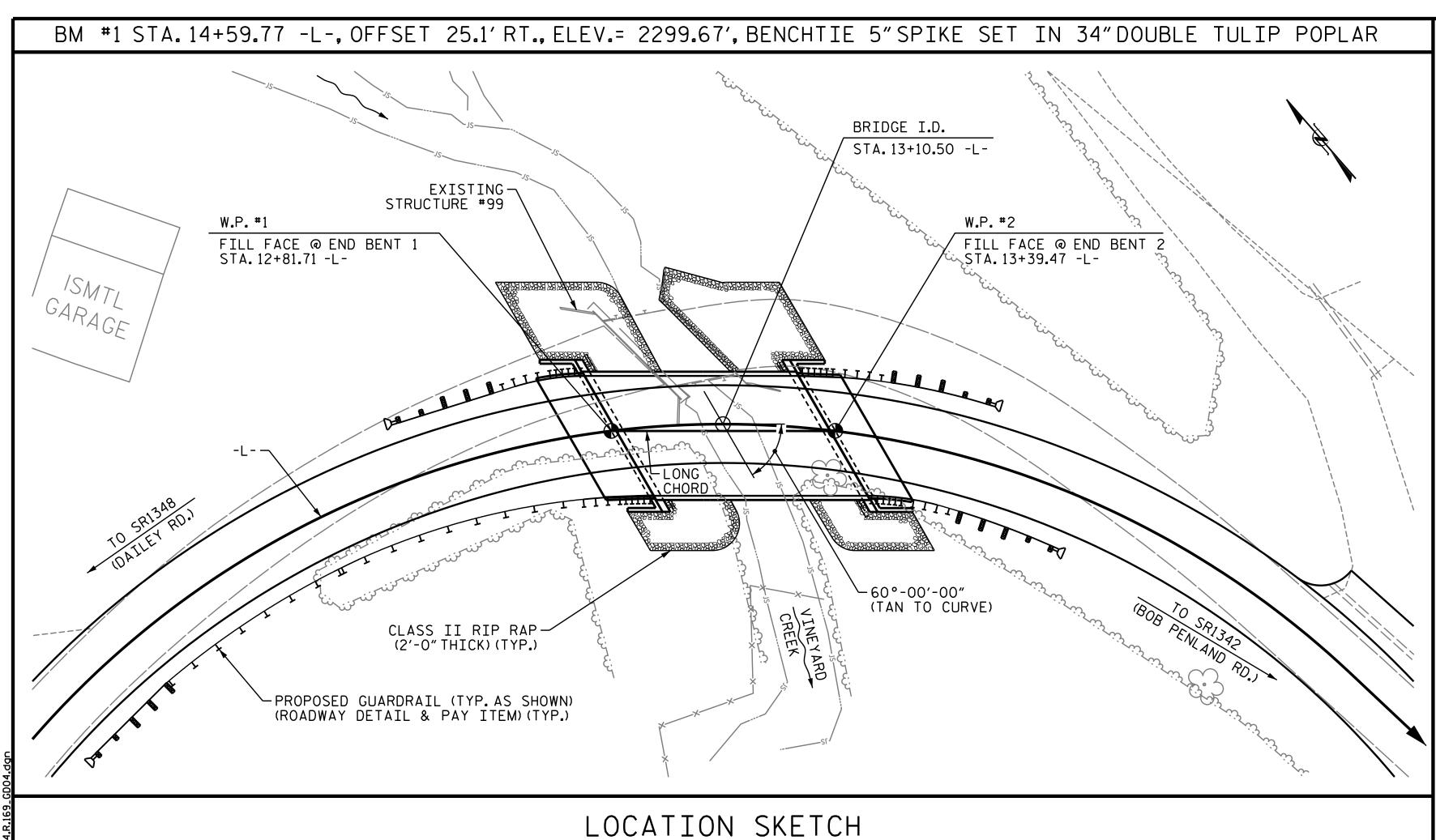
FOR BRIDGE ON SR 1168 OVER VINEYARD CREEK BETWEEN SR 1348 AND SR 1342

WSP USA Inc.
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DESIGNED BY: J. WHEATLEY DATE: 10/2021
DRAWN BY: M. HOBBS DATE: 10/2021
CHECKED BY: T. HARRIS DATE: 10/2021
DESIGN ENGINEER OF RECORD: T. HARRIS DATE: 10/2021

DRAWN BY:

CHECKED BY:



FOR UTILITY INFORMATION. SEE UTILITY PLANS AND SPECIAL PROVISIONS.

NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF A SINGLE SPAN @ 18'-6"WITH A CLEAR ROADWAY WIDTH OF 19'-0": TIMBER DECK WITH ASPHALT WEARING SURFACE ON TIMBER JOISTS WITH TIMBER CAPS, TIMBER POSTS AND END BENTS AND LOCATED UPSTREAM FROM PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY NOT POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED STRUCTURE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE".

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S-1 SHALL BE EXCAVATED FOR A DISTANCE OF 40 FT (LEFT) AND 31 FT. (RIGHT) OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 - EVALUATING SCOUR AT BRIDGES".

ASPHALT WEARING SURFACE IS INCLUDED IN THE ROADWAY QUANTITY ON ROADWAY PLANS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

	TOTAL BILL OF MATERIAL																
	REMOVAL OF EXISTING STRUCTURE @ STA.13+10.50 -L-	ASBESTOS ASSESSMENT	PILE EXCAVATION IN SOIL	PILE EXCAVATION NOT IN SOIL	EVCAAAITON	CLASS A CONCRETE	BRIDGE APPROACH SLABS @ STA. 13+10.50 -L-		PILE DRIVING EQUIPMENT SETUP FOR HP 12 × 53 STEEL PILES	HP STE	12 X 53 EL PILES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0"THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	3'- PRE C COF	O"X 1'-9" ESTRESSED ONCRETE RED SLABS
	LUMP SUM	LUMP SUM	LIN.FT.	LIN.FT.	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	No.	No.	LIN.FT.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	No.	LIN.FT.
SUPERSTRUCTURE							LUMP SUM					110.29			LUMP SUM	11	605.00
END BENT 1			60.0	35 . 0	LUMP SUM	24.1		2923	7	7	260		227	252			
END BENT 2			40.0	35.0	LUMP SUM	24.1		2923	7	7	245		220	244			
TOTAL	LUMP SUM	LUMP SUM	100.0	70.0	LUMP SUM	48.2	LUMP SUM	5846	14	14	505	110.29	447	496	LUMP SUM	11	605.00

PROJECT NO. 17BP.14.R.169 CLAY _ COUNTY

STATION: 13+10.50 -L-

SHEET 4 OF 4

DOCUMENT NOT CONSIDERED FINA JNLESS ALL SIGNATURES COMPLETE

Thomas Harris 111/23/2021

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING

FOR BRIDGE ON SR 1168 OVER VINEYARD CREEK BETWEEN SR 1348 AND SR 1342

		SHEET NO.				
0.	BY:	DATE:	NO.	BY:	DATE:	S-4
0			3			TOTAL SHEETS
2			4			17

WSP USA Inc. 434 FAYETTEVILLE STREET SUITE 1500 RALEIGH, NC 27601 TEL: 1.919.836.4040 LICENSE NO. F-0165

 J. WHEATLEY
 DATE : 10/2021

 M. HOBBS
 DATE : 10/2021

 T. HARRIS
 DATE : 10/2021

 DESIGN ENGINEER
OF RECORD: ____ _ DATE : 10/2021

LOAD

RATING

26.923

26.923

26.923

26.923

26.923

26.923

26.923

EL

EL

EL

EL

EL

EL

0.659

0.659

0.659

0.659

0.659

0.659

0.659

26.923 0.659

1.92

1.85

1.81

1.67

1.58

1.52

1.55

1.44

55′

55′

55′

55′

55′

55′

55′

55′

EL

EL

EL

EL

10.769

10.769

10.769

10.769

10.769

10.769

10.769

0.80

0.80

0.80

0.80

0.80

0.80

0.80

10.769 | 0.80 | 0.249 | **1.12** |

0.249

0.249

0.249

0.249

0.249

0.249

0.249

1.45

1.46

1.21

1.23

1.28

1.21

1.14

55′

55′

55′

55′

55′

55′

55′

26.923

26.923

26.923

26.923

26.923

26.923

26.923

26.923

EL

EL

EL

EL

LOAD FACTORS:

	DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
	LOAD RATING FACTORS	STRENGTH I	1.25	1.50
		SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

- (#) CONTROLLING LOAD RATING
- 1 DESIGN LOAD RATING (HL-93)
- 2 DESIGN LOAD RATING (HS-20)
- 3 LEGAL LOAD RATING **
 - GIRDER LOCATION

** SEE CHART FOR VEHICLE TYPE

- I INTERIOR GIRDER
- EL EXTERIOR LEFT GIRDER
- ER EXTERIOR RIGHT GIRDER

PROJECT NO. 17BP.14.R.169 CLAY COUNTY

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD (NON-INTERSTATE TRAFFIC)

REVISIONS SHEET NO. S-5 NO. BY: DATE: BY: DATE:

STATION: 13+10.50 -L-

DOCUMENT NOT CONSIDERED FINAL JNLESS ALL SIGNATURES COMPLETE Thomas Harris 11/23/2021

LRFR SUMMARY FOR SPAN 'A'

ASSEMBLED BY: J. WHEATLEY DATE: 10/2021 CHECKED BY: T. HARRIS DATE: 10/2021 DESIGN ENGINEER
OF RECORD: T. HARRIS __ DATE : 10/2021

DRAWN BY : CVC 6/10 CHECKED BY : DNS 6/10

33.000

33.075

41.600

42.000

42.000

43.000

45.000

45.000

3

TNAGRIT3

TNT4A

TNT6A

TNT7A

TNT7B

TNAGRIT4

TNAGT5A

TNAGT5B

47.984

48.451

50.478

51.576

53.827

52.158

51.134

1.116 50.224

0.249

0.249

0.249

0.249

0.249

0.249

0.249

0.249

1.4

2.12

2.14

1.77

1.79

1.87

1.77

1.63

55′

55′

55′

55′

55′

55′

55′

1.454

1.465

1.213

1.228

1.282

1.213

1.136

DRAWN BY: DGE 5/09 CHECKED BY: BCH 6/09

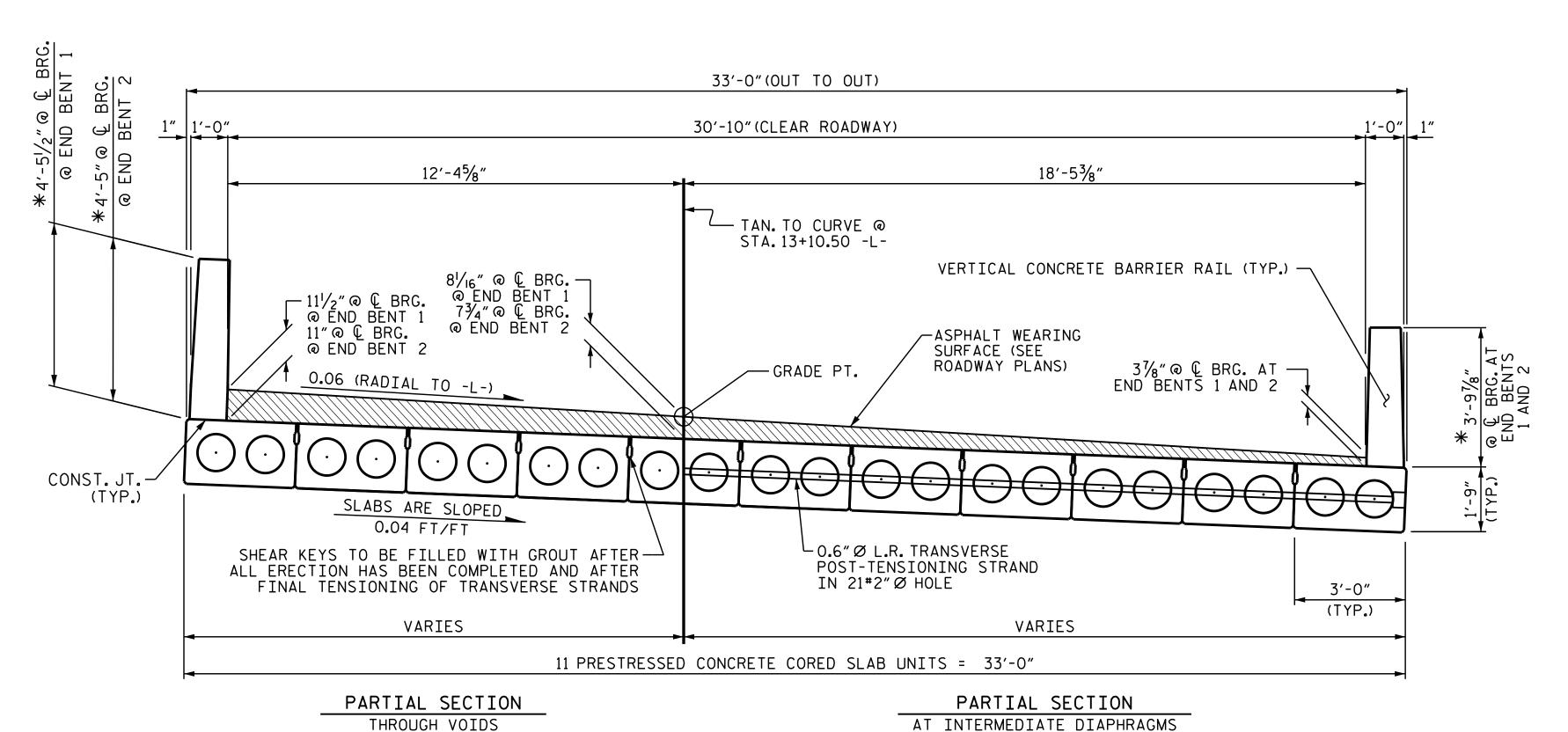
DESIGN ENGINEER _ OF RECORD: ____

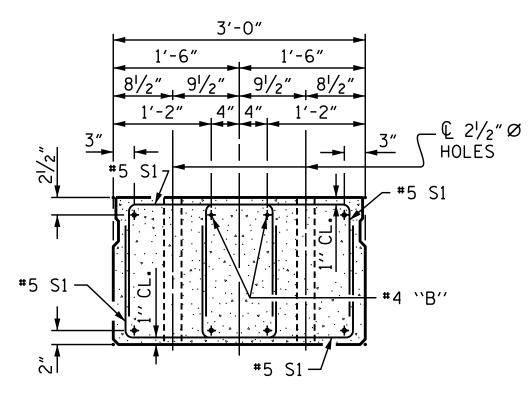
REV. 9/14

_ DATE : 10/2021

ASSEMBLED BY: J. WHEATLEY DATE: 10/2021 CHECKED BY: T. HARRIS DATE: 10/2021

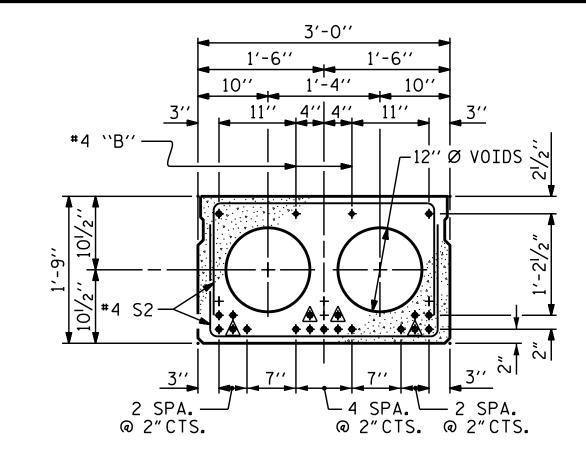
T. HARRIS





END ELEVATION

SHOWING PLACEMENT OF DOUBLE STIRRUPS AND LOCATION OF 21/2" Ø HOLES. (STRAND LAYOUT NOT SHOWN.) INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.

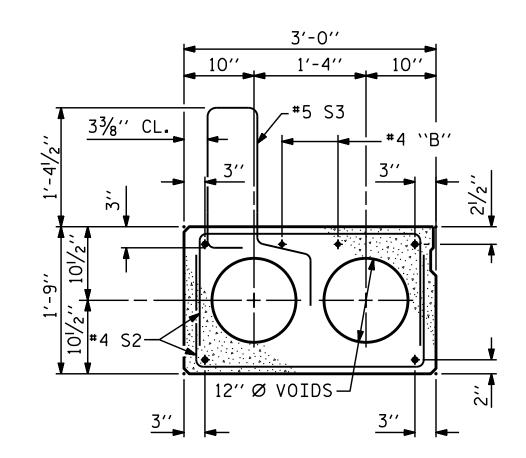


INTERIOR SLAB SECTION (19 STRANDS REQUIRED)

0.6" Ø LOW RELAXATION STRAND LAYOUT

BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 6'-0" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

DEBONDING LEGEND



EXT. SLAB SECTION

(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTIONS

PROJECT NO. 17BP.14.R.169 CLAY COUNTY

13+10.50 -L-STATION:

SHEET 1 OF 3

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AS M. H

Thomas Harris 11/23/2021

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

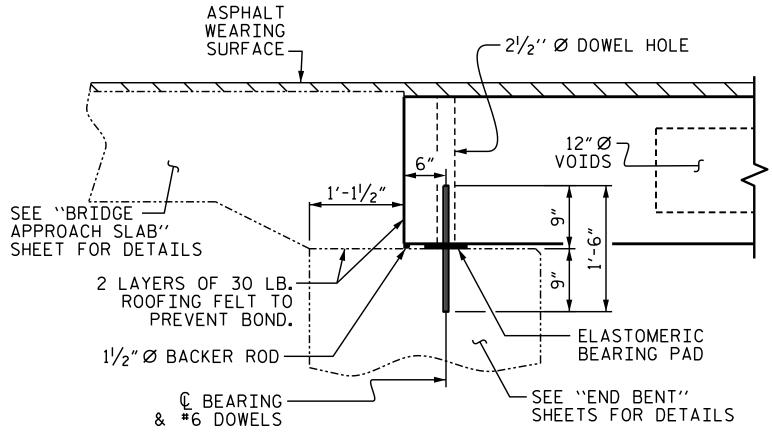
STANDARD

3'-0" X 1'-9" PRESTRESSED CONCRETE CORED SLAB UNIT 60° SKEW

REVISIONS SHEET NO. S-6 NO. BY: DATE: DATE: BY:

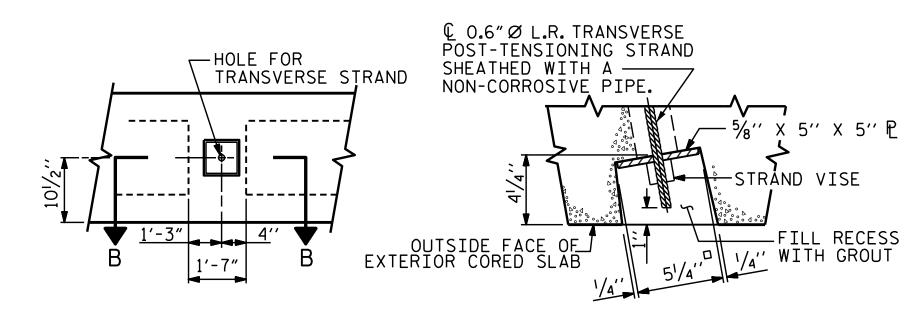
TYPICAL SECTION

* - THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.



SECTION AT END BENT

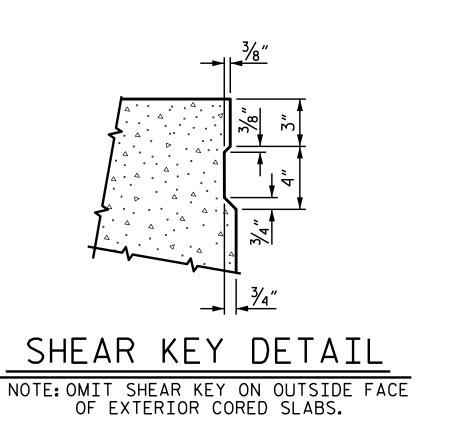
PERMITTED THREADED INSERT CAST IN OUTSIDE FACE OF EXTERIOR UNIT AND RECESSED 3/8". SIZE TO BE DETERMINED BY CONTRACTOR. THREADED INSERT DETAIL

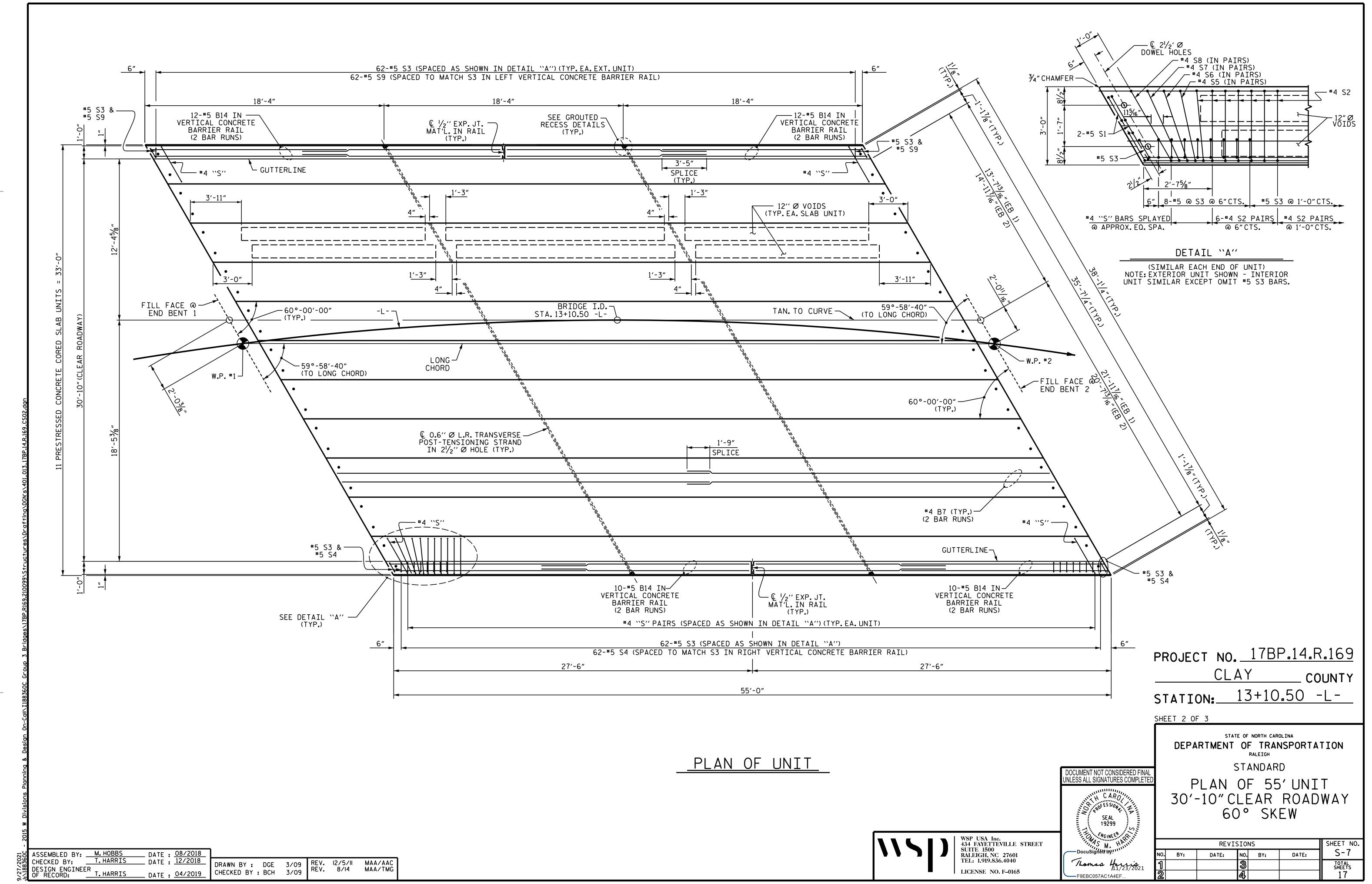


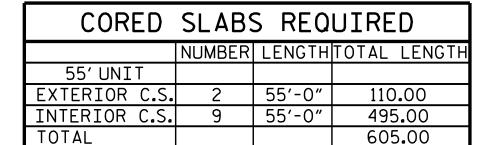
ELEVATION VIEW

SECTION B-B

GROUTED RECESS AT END OF POST-TENSIONED STRAND OF CORED SLABS







BI	BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL									
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT				
	55' UNIT									
 ₩B14	88	88	#5	STR	15′-6″	1423				
* S4	64	64	#5	2	7′-2″	478				
* S9	64	64	#5	2	8'-6"	567				
* EPOX	Y COATED REINFORCING STEEL			LBS.		2468				
CLASS AA CONCRETE CU.YDS. 1										
TOTAL										

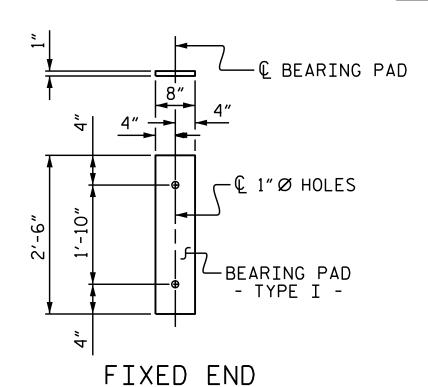
SECTION S-S

AT DAM IN OPEN JOINT

(THIS IS TO BE USED ONLY

WHEN SLIP FORM IS USED)

END VIEW



(TYPE I - 22 REQ'D)

ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.

1'-0"

DEAD LOAD DEFLECTION A	ND CAMBER
	3'-0" × 1'-9"
55' CORED SLAB UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	11/2"
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	3/8″ ♦
FINAL CAMBER	1 ¹ / ₈ " ∤

** INCLUDES FUTURE WEARING SURFACE

Ĺ 1/2"EXP.JT.MAT'L HELD IN

PLACE WITH GALVANIZED NAILS.

WHEN SLIP FORM IS USED)

CHAMFER.

CONST.

CHAMFER

(NOTE: OMIT EXP. JT. MAT'L.

BAR TYPES 6" 0 4 6" S7 2'-11' ALL BAR DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL FOR ONE 55' CORED SLAB UNIT											
EXTERIOR UNIT INTERIOR UNIT											
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT				
В7	4	#4	STR	28'-3"	75	28'-3"	75				
S1	8	#5	3	4′-6″	38	4'-6"	38				
S2	112	#4	3	5′-4″	399	5′-4″	399				
÷ S3	64	# 5	1	5′-7″	373						
S5	4	#4	3	5′-5″	14	5′-5″	14				
S6	4	#4	3	5′-6″	15	5′-6″	15				
S 7	4	#4	3	5′-7″	15	5′-7″	15				
S8	4	#4	3	5′-9″	15	5′-9″	15				
REINFO	RCING	STEEL	LBS	Š.	571		571				
	Y COATE										
	FORCINO		LB:	5.	373						
6500 F	P.S.I.CO	NCRETE	CU. YDS) _a	8.0		8.0				
0.6"Ø	L.R. STR	ANDS	No),	19		19				

GUTTERLINE ASPI	HALT THICKNESS & RAI	L HEIGHT
	ASPHALT OVERLAY THICKNESS	RAIL HEIGHT
	@ MID-SPAN	@ MID-SPAN
LEFT GUTTER	91/16"	4'-3 ^l / ₁₆ "
RIGHT GUTTER	11/2"	3'-71/2"

SIDE VIEW

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2^{1}/2^{*}$ Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS. LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

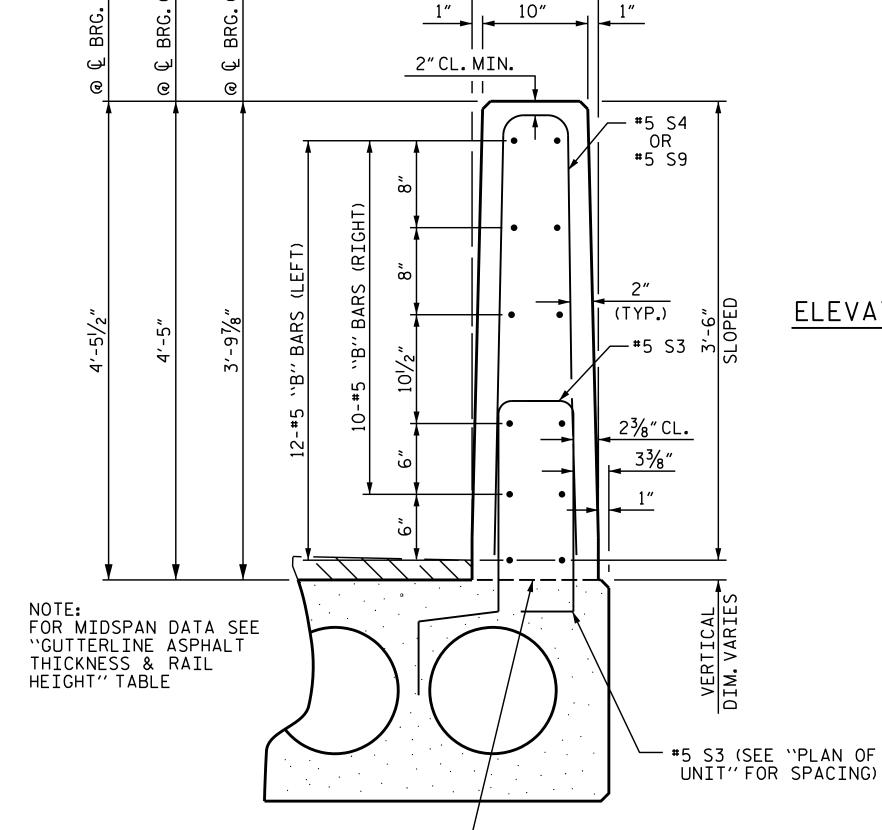
THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

UNIT

55' UNITS



CONST. JT. —

ELEVATION AT EXPANSION JOINTS 2'-0" 4-#5 S3 6" #5 S3 & S4 OR S9 4-#5 S3 & S4 OR | & S4 OR S9 @ S9 @ 6"CTS. 6"CTS. FIELD BEND — "B" BARS \|FIELD CUT| FIELD CUT-#5 S4 OR #5 S9 OR #5 S9 FIELD-CUT #5 S4 OR #5 S9 CONST.JT.

0.6"Ø L.R. 0.217 (SQUARE INCHES) ULTIMATE STRENGTH (LBS.PER STRAND) 58,600 APPLIED PRESTRESS (LBS.PER STRAND) 43,950

WSP USA Inc. 434 FAYETTEVILLE STREET SUITE 1500 RALEIGH, NC 27601 TEL: 1.919.836.4040

LICENSE NO. F-0165

GRADE 270 STRANDS

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NGINEER HAR

Thomas Harris 11/23/2021

PROJECT NO. 17BP.14.R.169 CLAY COUNTY 13+10.50 -L-

PSI

4900

CONCRETE RELEASE STRENGTH

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

3'-0'' X 1'-9'' PRESTRESSED CONCRETE CORED SLAB UNIT 60° SKEW

REVISIONS SHEET NO. S-8 NO. BY: DATE: BY: DATE:

END OF RAIL DETAILS

ASSEMBLED BY: J. WHEATLEY DATE: 10/2021 CHECKED BY: T. HARRIS DATE: 10/2021 DRAWN BY: DGE 5/09 DESIGN ENGINEER
OF RECORD: T. HARRIS _ DATE : 10/2021

REV. 5/18 MAA/THC CHECKED BY : BCH 6/09

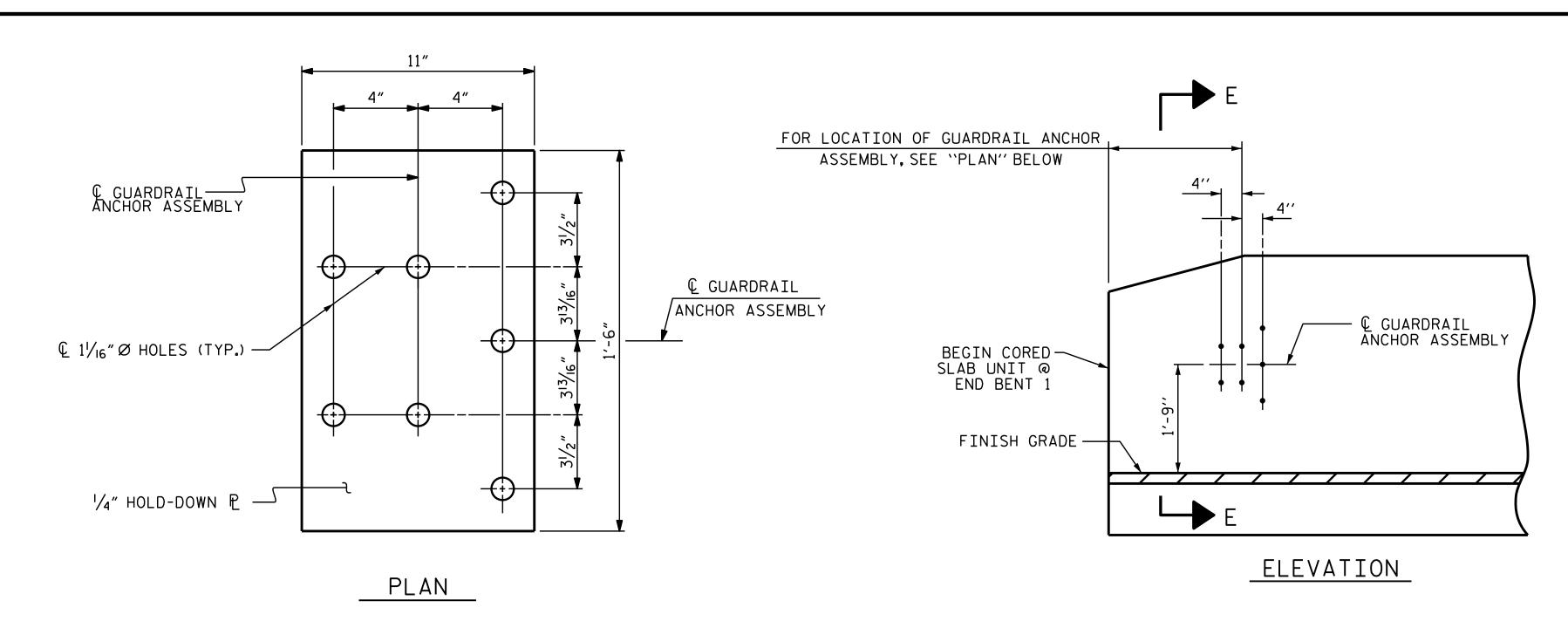
VERTICAL CONCRETE BARRIER RAIL SECTION

STD. NO. 21" PCS3_33_60S

ASSEMBLED BY: J. WHEATLEY DATE: 10/2021 CHECKED BY: T. HARRIS DATE: 10/2021

__ DATE : 10/2021

DESIGN ENGINEER
OF RECORD: T. HARRIS



NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

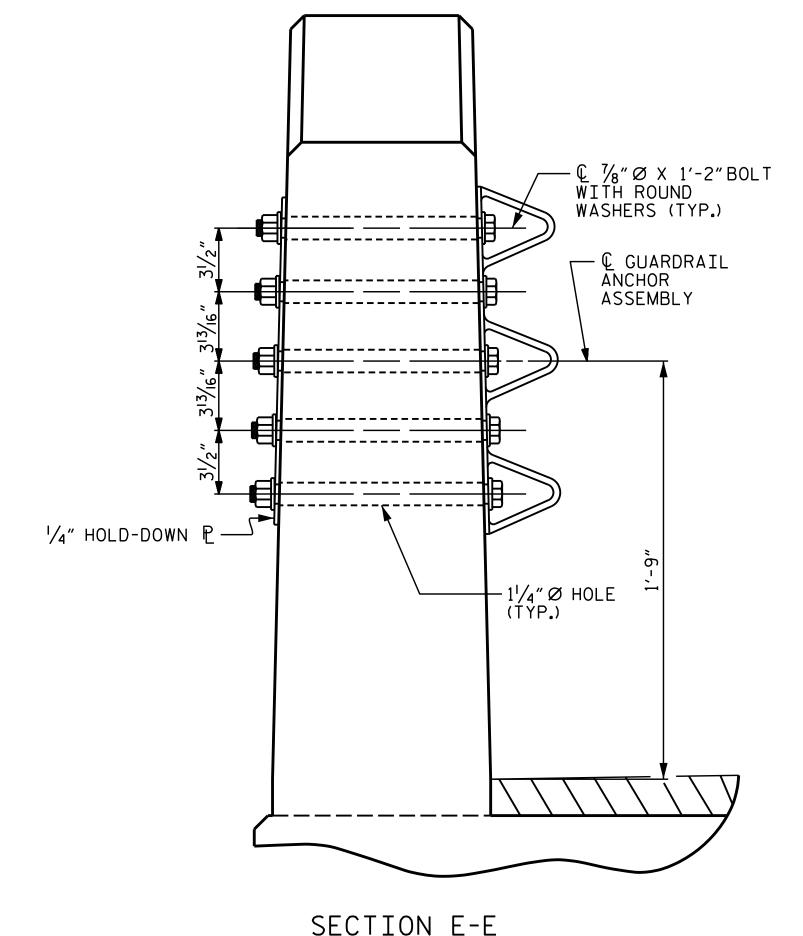
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

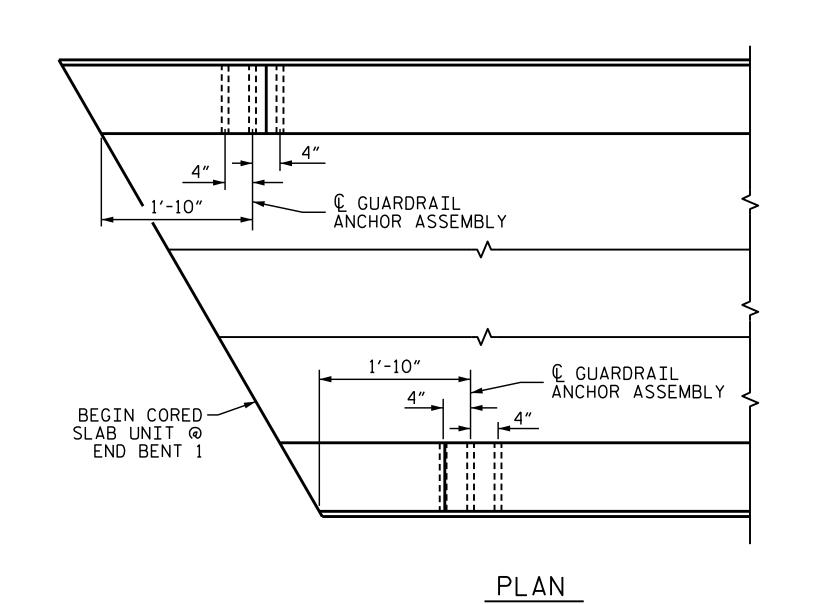


SECTION E-E GUARDRAIL ANCHOR ASSEMBLY DETAILS

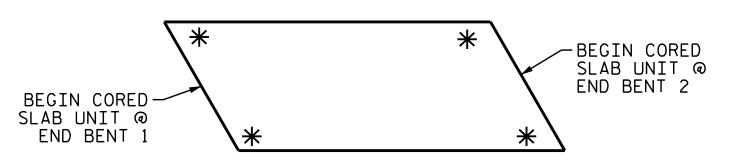
MAA/THG

DRAWN BY : MAA 5/10

CHECKED BY : GM 5/10



LOCATION OF ANCHORS FOR GUARDRAIL



SKETCH SHOWING POINTS OF ATTACHMENT

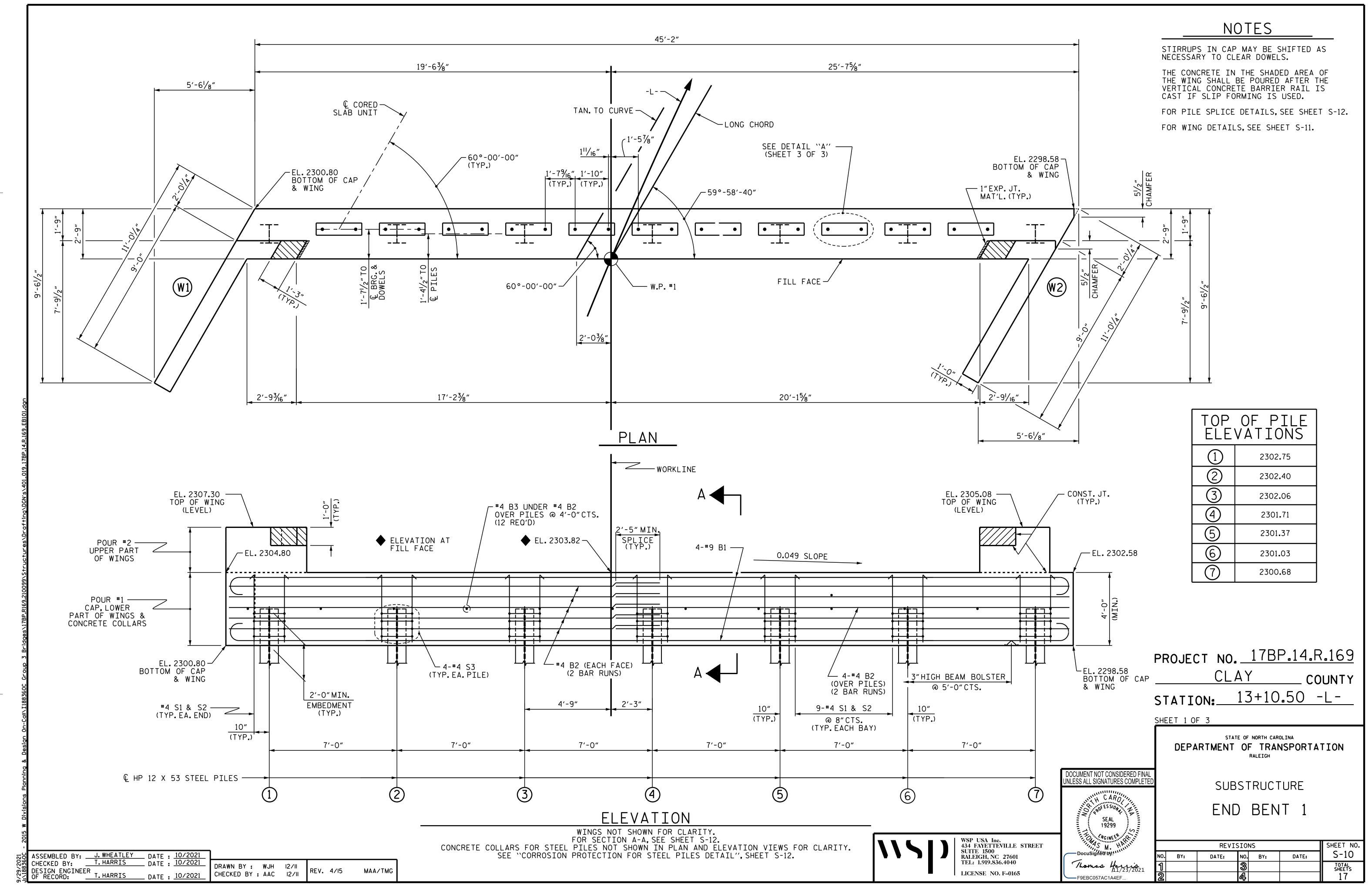
* DENOTES GUARDRAIL ANCHOR ASSEMBLY

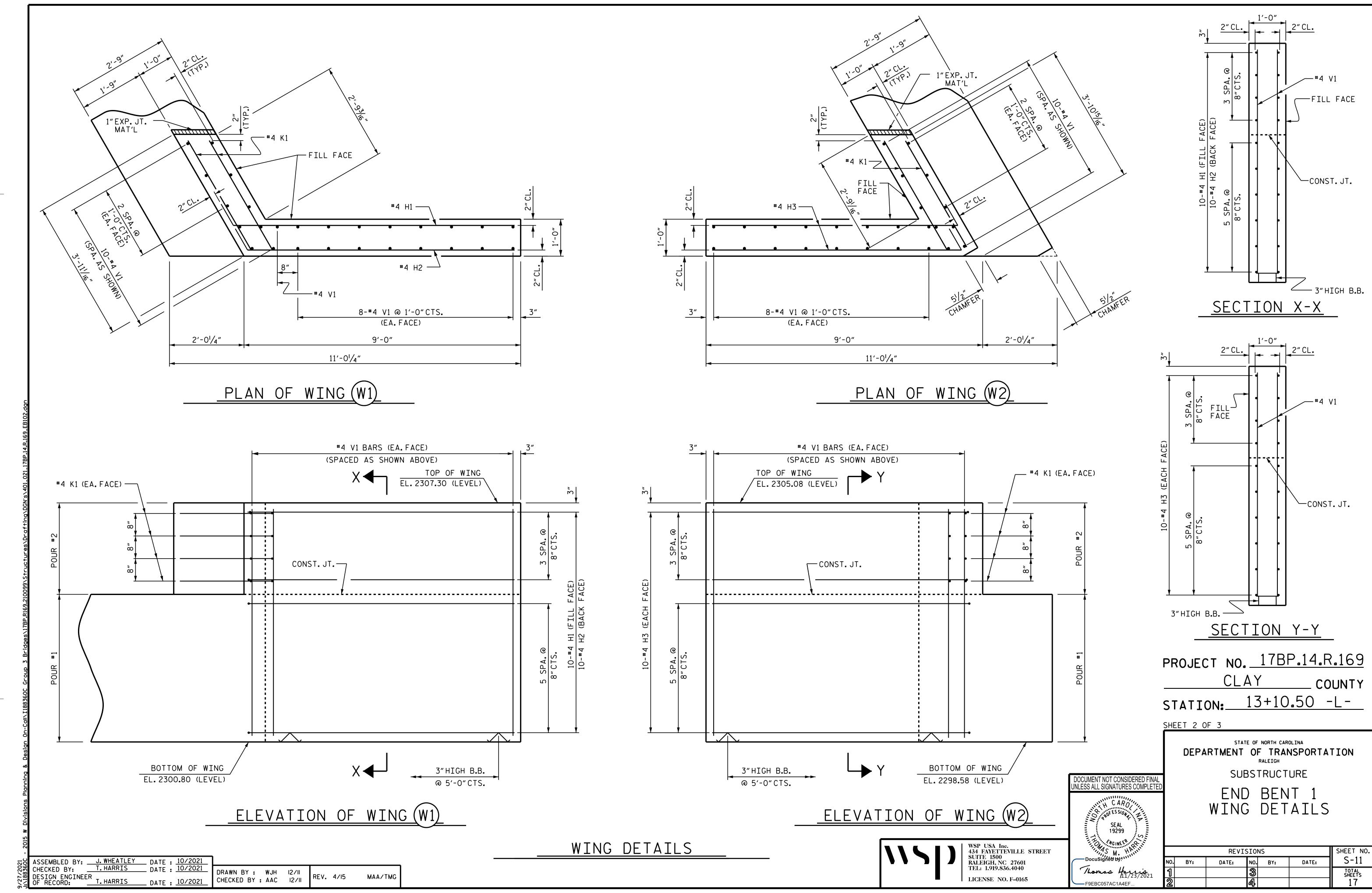
PROJECT NO. 17BP.14.R.169 CLAY COUNTY STATION: 13+10.50 -L-

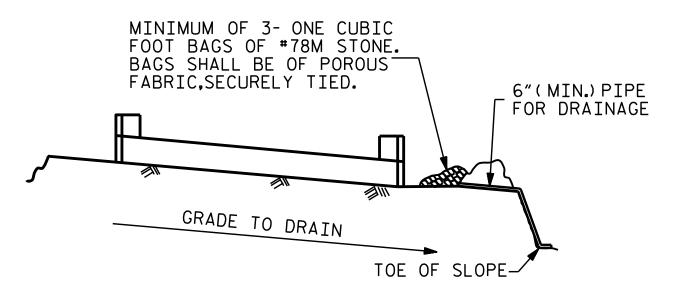
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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD GUARDRAIL ANCHORAGE DETAILS FOR VERTICAL CONCRETE BARRIER RAIL

	SHEET NO.									
BY:	DATE:	NO.	BY:	DATE:	S-9					
		3			TOTAL SHEETS					
		4			17					
		ST	D. NC	GRA3						





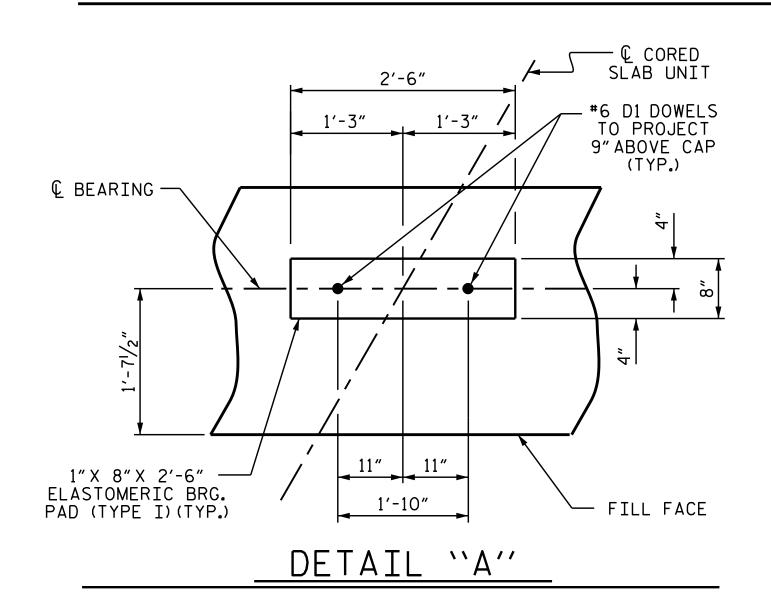


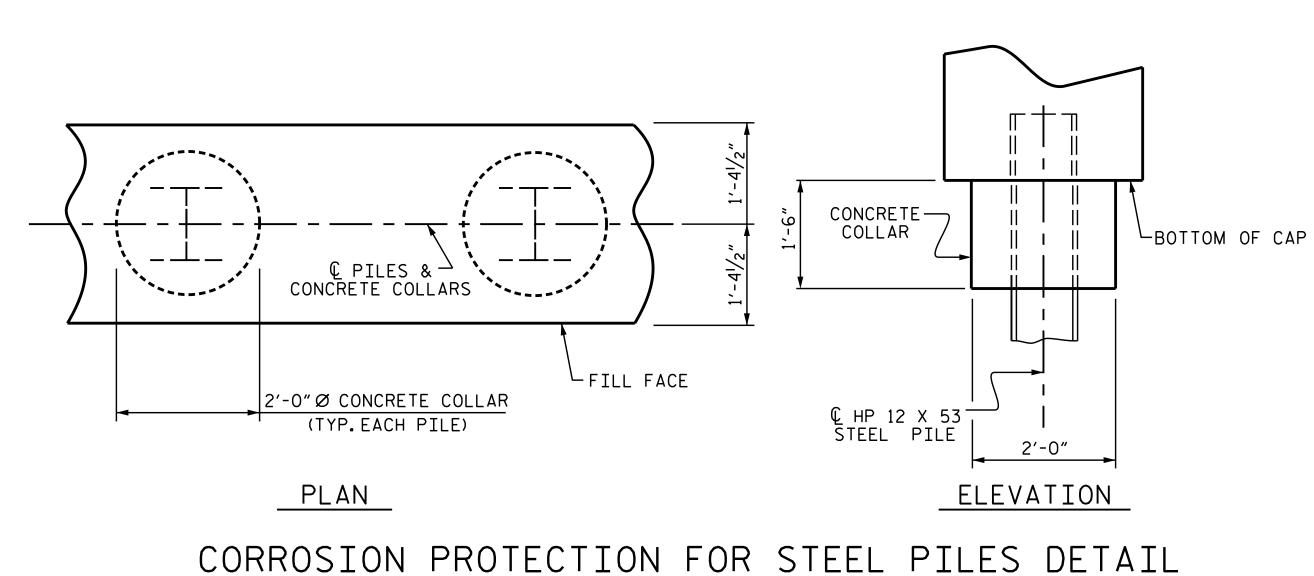
BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

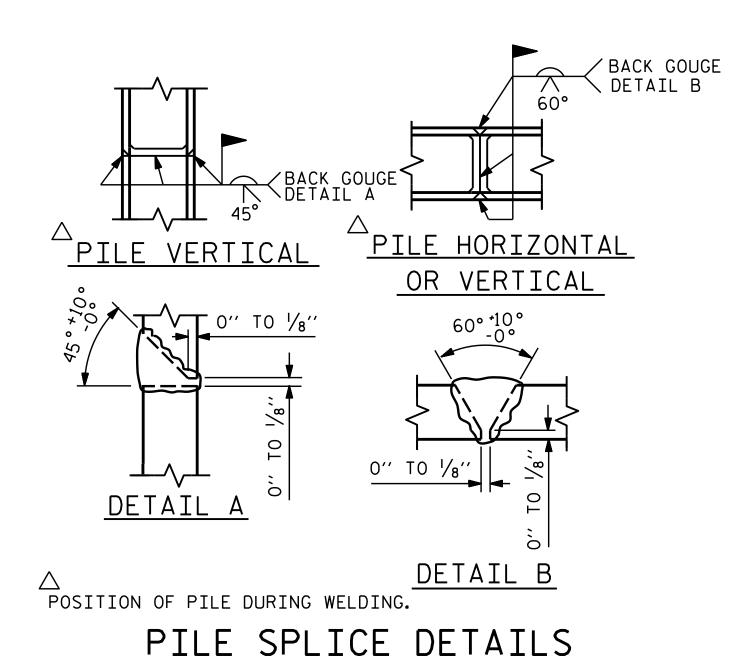
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

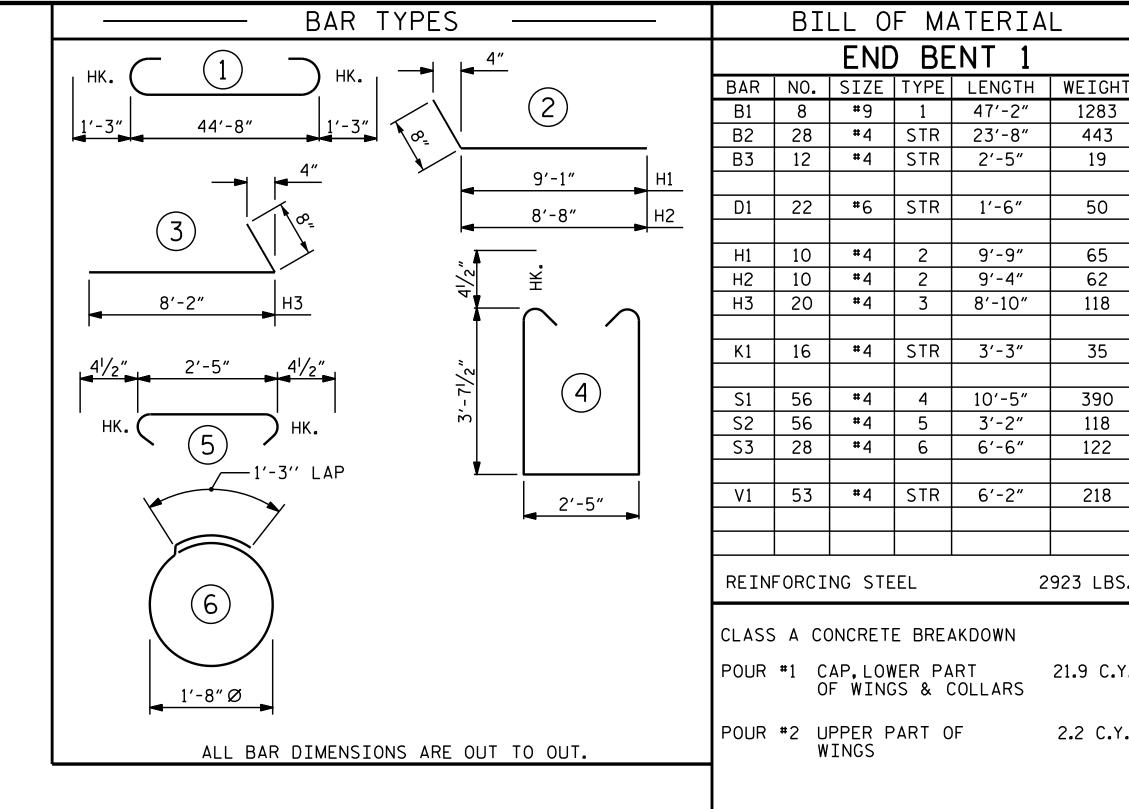
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT









	В3	12	#4	STR	2′-5″	19		
-								
	D1	22	#6	STR	1′-6″	50		
-								
	H1	10	#4	2	9'-9"	65		
	H2	10	#4	2	9'-4"	62		
	Н3	20	#4	3	8'-10"	118		
	K1	16	#4	STR	3′-3"	35		
	S1	56	#4	4	10'-5"	390		
	S2	56	#4	5	3'-2"	118		
	S3	28	#4	6	6'-6"	122		
	V1	53	#4	STR	6′-2″	218		
	REINFORCING STEEL 2923 LBS.							
	,	J U I		- -	_			
	CI V C C	۸ ۲۸	NICDETI	E RDE	AKDOWN			
	CLASS	ACC	MUNEII	L DNE	ANDOWN			

BILL OF MATERIAL

END BENT

#4 STR 23'-8"

47'-2"

443

21.9 C.Y.

2.2 C.Y.

NO: 7

#9 |

28

TOTAL CLASS A CONCRETE 24.1 C.Y. END BENT 1

OF WINGS & COLLARS

HP 12 X 53 STEEL PILES LIN.FT.= 260.0 NO: 7

PILE EXCAVATION IN SOIL LIN. FT.= 60.0

WINGS

PILE EXCAVATION NOT IN SOIL LIN. FT.= 35.0

PILE DRIVING EQUIPMENT SETUP FOR HP 12 X 53 STEEL PILES

1'-7¹/2" FILL FACE 2"CL. r#4.S2 4-#9 B1 -4-#4 B2 @ 4" CTS. 1-#4 B2 — OVER PILES EA.FACE #4 S1 ____ 2-**#**9 B1 2"CL.(TYP.)-2-#9 B1 € HP 12 X 53 STEEL PILE— — 3" HIGH B.B. $1'-4^{1}/2''$ $1'-4^{1}/2''$ 2'-9"

€ #6 D1 DOWEL

SECTION A-A

(CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL.")

PROJECT NO. 17BP.14.R.169 CLAY COUNTY 13+10.50 -L-STATION:_ SHEET 3 OF 3

DOCUMENT NOT CONSIDERED FINAL JNLESS ALL SIGNATURES COMPLETE

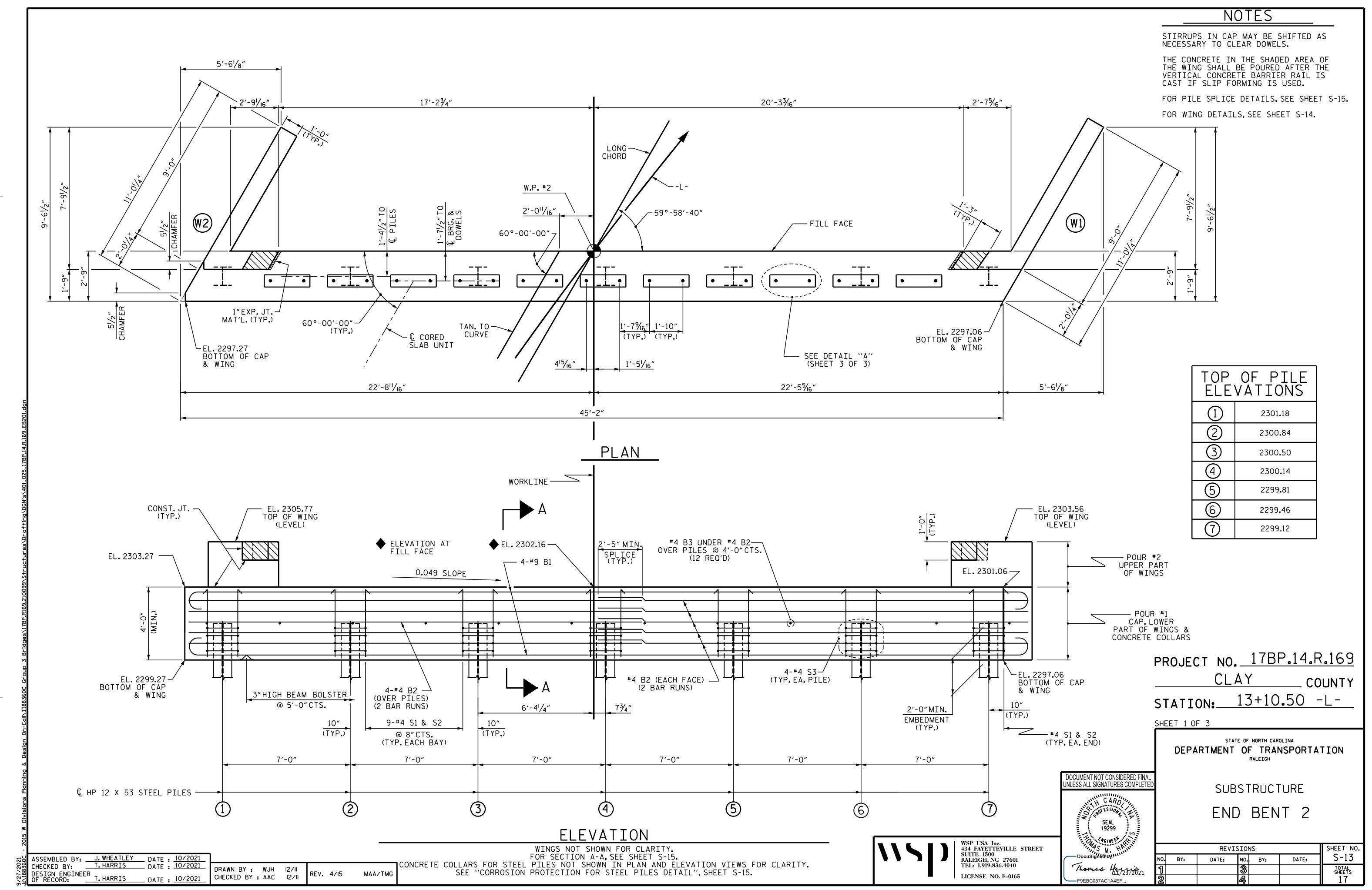
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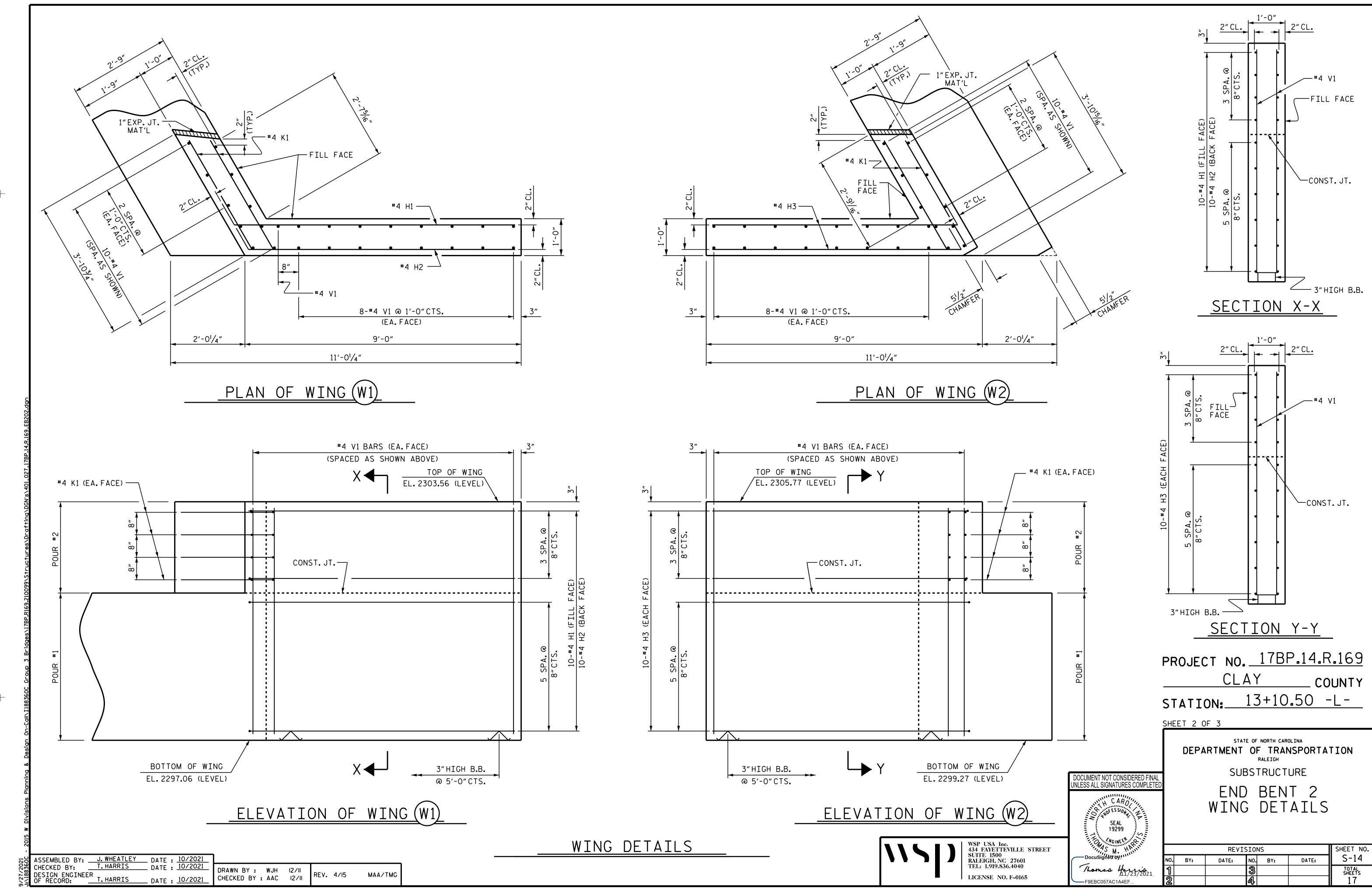
Thomas Harris 11/23/2021

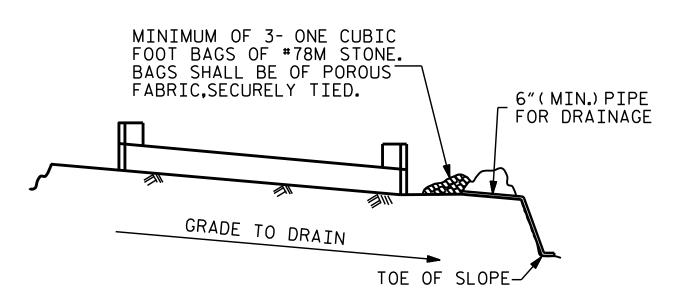
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SUBSTRUCTURE

> END BENT 1 DETAILS

SHEET NO. REVISIONS S-12 NO. BY: DATE: DATE: BY:







BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

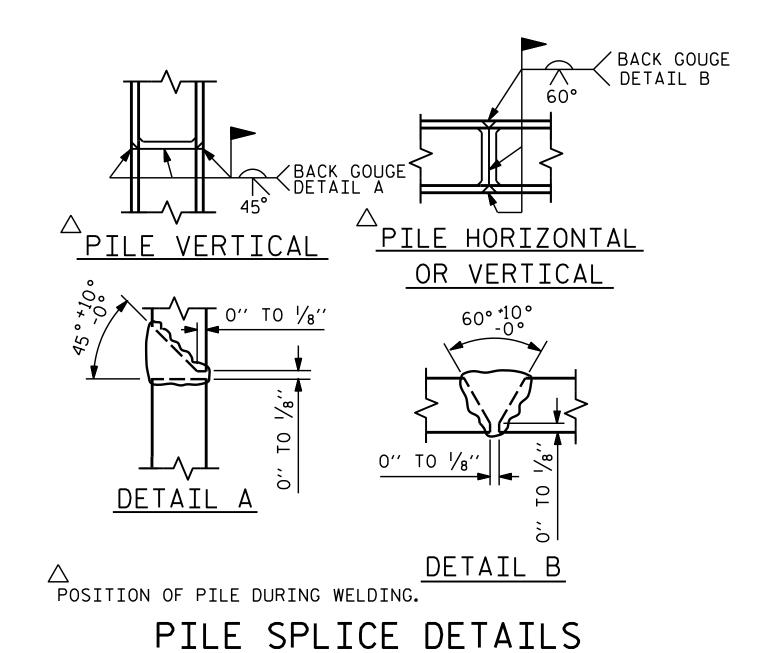
1'-10"

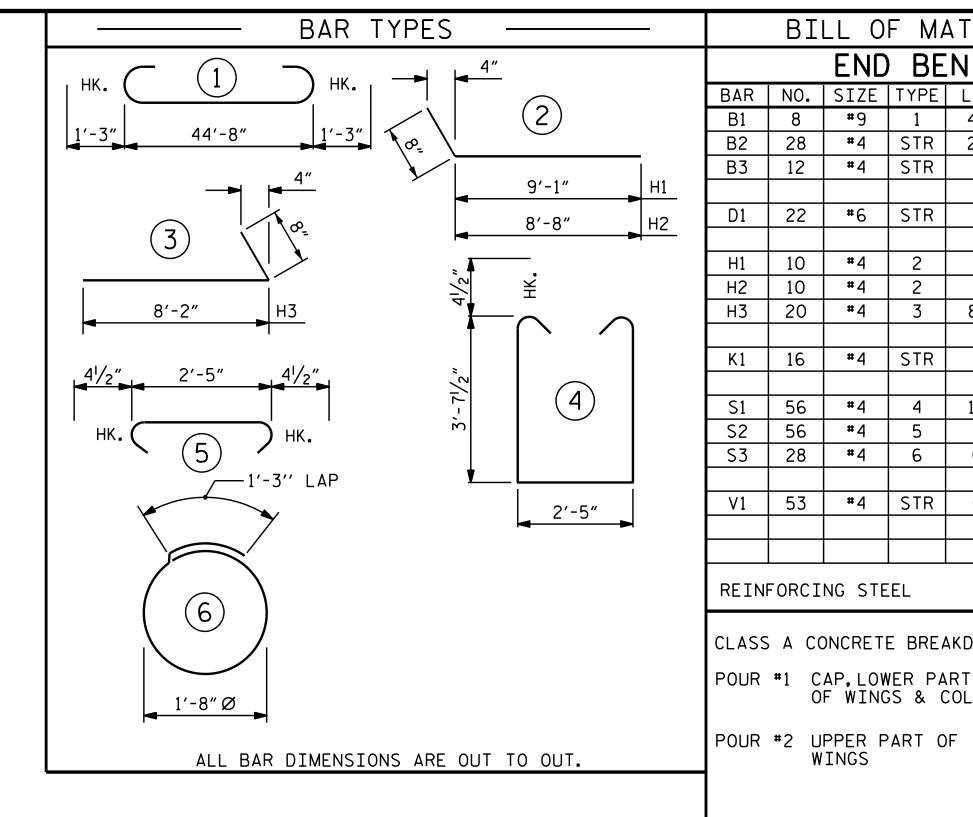
11" 11"

1" X 8" X 2'-6" ¬

ELASTOMERIC BRG.

PAD (TYPE I) (TYP.)





H1	10	#4	2	9'-9"	65			
H2	10	#4	2	9'-4"	62			
Н3	20	#4	3	8'-10"	118			
K1	16	#4	STR	3′-3″	35			
S1	56	#4	4	10′-5″	390			
S2	56	#4	5	3'-2"	118			
S3	28	#4	6	6′-6″	122			
V1	53	#4	STR	6′-2″	218			
REINFORCING STEEL 2923 LBS.								
CLASS A CONCRETE BREAKDOWN								

BILL OF MATERIAL

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

#9 |

#4 | STR |

#6 | STR |

B2

В3

D1

28

1 22 1

END BENT 2

#4 | STR | 23'-8"

47′-2"

2′-5″

1′-6″

443

19

50

21.9 C.Y.

2.2 C.Y.

24.1 C.Y.

LIN. FT.= 245.0

LIN. FT.= 40.0

TOTAL CLASS A CONCRETE

OF WINGS & COLLARS

END BENT 2 HP 12 X 53 STEEL PILES

WINGS

NO: 7

PILE EXCAVATION

IN SOIL

PILE EXCAVATION

NOT IN SOIL LIN. FT.= 35.0

PILE DRIVING EQUIPMENT SETUP FOR HP 12 X 53 STEEL PILES

NO: 7

 BEARING -#6 D1 DOWELS 1'-3" 1'-3" 9" ABOVE CAP 2'-6" € CORED SLAB UNIT DETAIL "A" FILL FACE CONCRETE-COLLAR BOTTOM OF CAP € PILES & → CONCRETE COLLARS 2'-0" Ø CONCRETE COLLAR © HP 12 X 53 TEEL PILE (TYP.EACH PILE) ELEVATION PLAN

FILL FACE

CORROSION PROTECTION FOR STEEL PILES DETAIL

WSP USA Inc. 434 FAYETTEVILLE STREET SUITE 1500 RALEIGH, NC 27601 TEL: 1.919.836.4040 LICENSE NO. F-0165

€ #6 D1 DOWEL

-4-#4 B2 @ 4" CTS.

OVER PILES

2-#9 B1

— 3" HIGH B.B.

┌#4 S2 ந்

1'-71/2"

2" CL.

1'-41/2" 1'-41/2"

2'-9"

SECTION A-A

(CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL.")

#4 B3 —

#4 S1 _____

© HP 12 X 53 STEEL PILE—

FILL FACE

4-**#**9 B1

2-#9 B1

2"CL.(TYP.)-

1-#4 B2 EA.FACE

PROJECT NO. 17BP.14.R.169 CLAY COUNTY 13+10.50 -L-STATION:_

SHEET 3 OF 3

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETE

Thomas Harris 11/23/2021

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

END BENT 2 DETAILS

SHEET NO. REVISIONS S-15 NO. BY: DATE: DATE: BY:

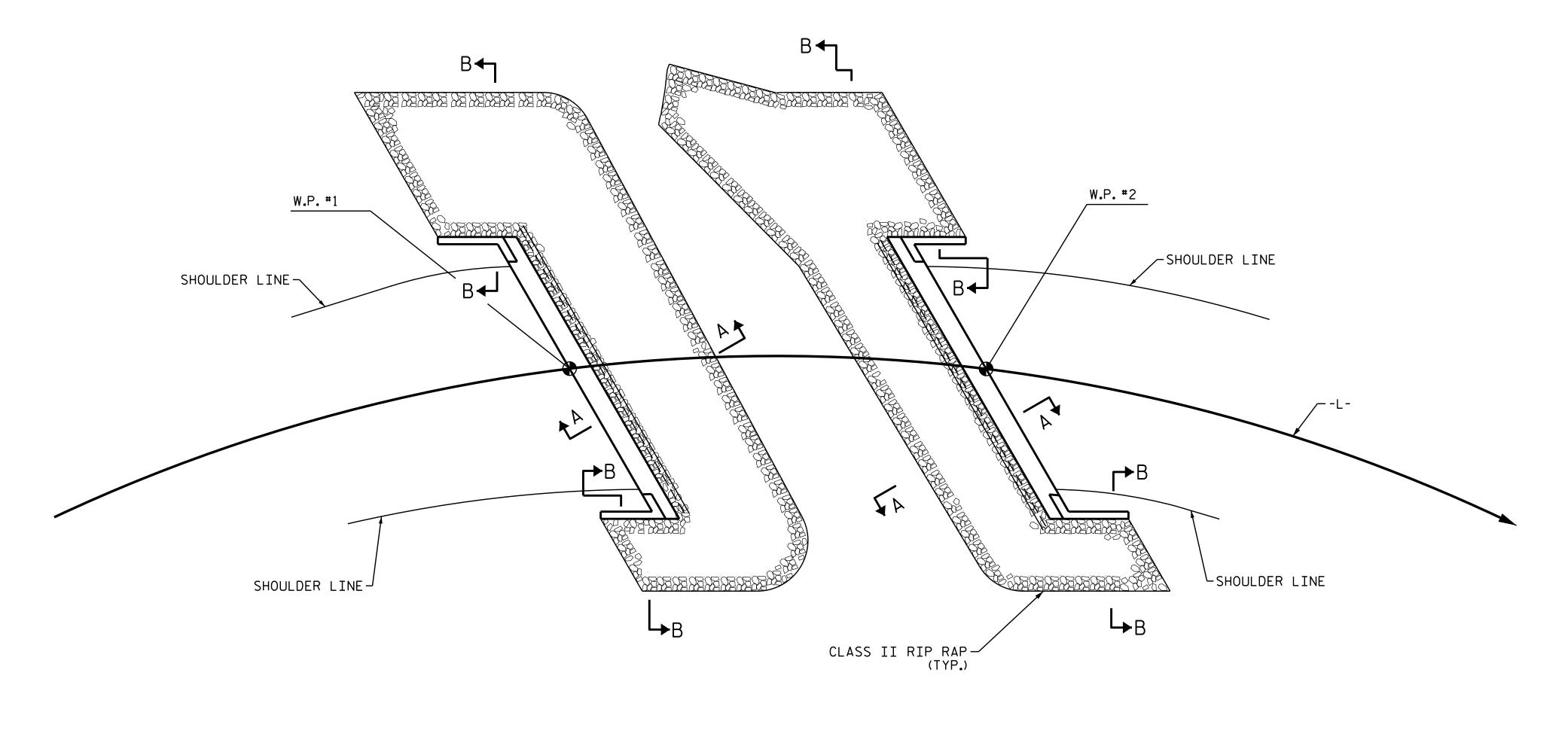
ASSEMBLED BY: J. WHEATLEY DATE: 10/2021 CHECKED BY: T. HARRIS DATE: 10/2021 DRAWN BY: WJH 12/11 DESIGN ENGINEER
OF RECORD: T. HARRIS DATE: 10/2021 REV. 4/17 MAA/THG CHECKED BY : AAC 12/11

DESIGNED BY: J. WHEATLEY DATE: 10/2021

DRAWN BY: M. HOBBS DATE: 10/2021

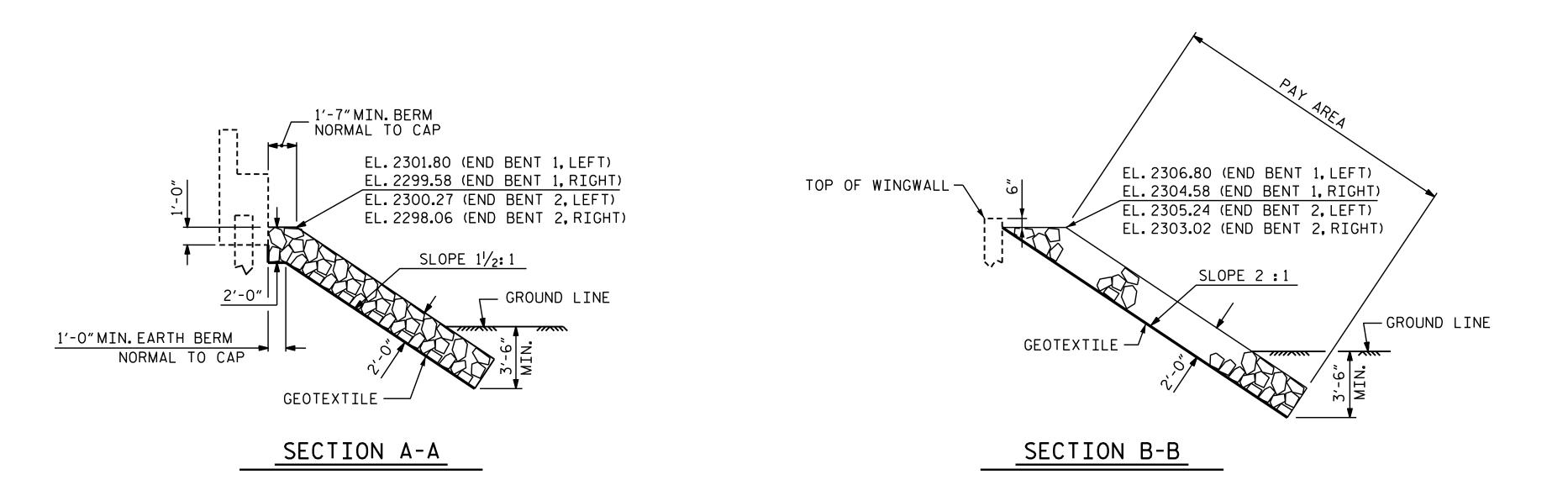
CHECKED BY: T. HARRIS DATE: 10/2021

DESIGN ENGINEER OF RECORD: T. HARRIS DATE: 10/2021



ESTIMATED QUANTITIES							
BRIDGE @ STA.13+10.50 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE					
	TONS	SQUARE YARDS					
END BENT 1	227	252					
END BENT 2	220	244					

PLAN



PROJECT NO. 17BP.14.R.169

CLAY COUNTY

STATION: 13+10.50 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

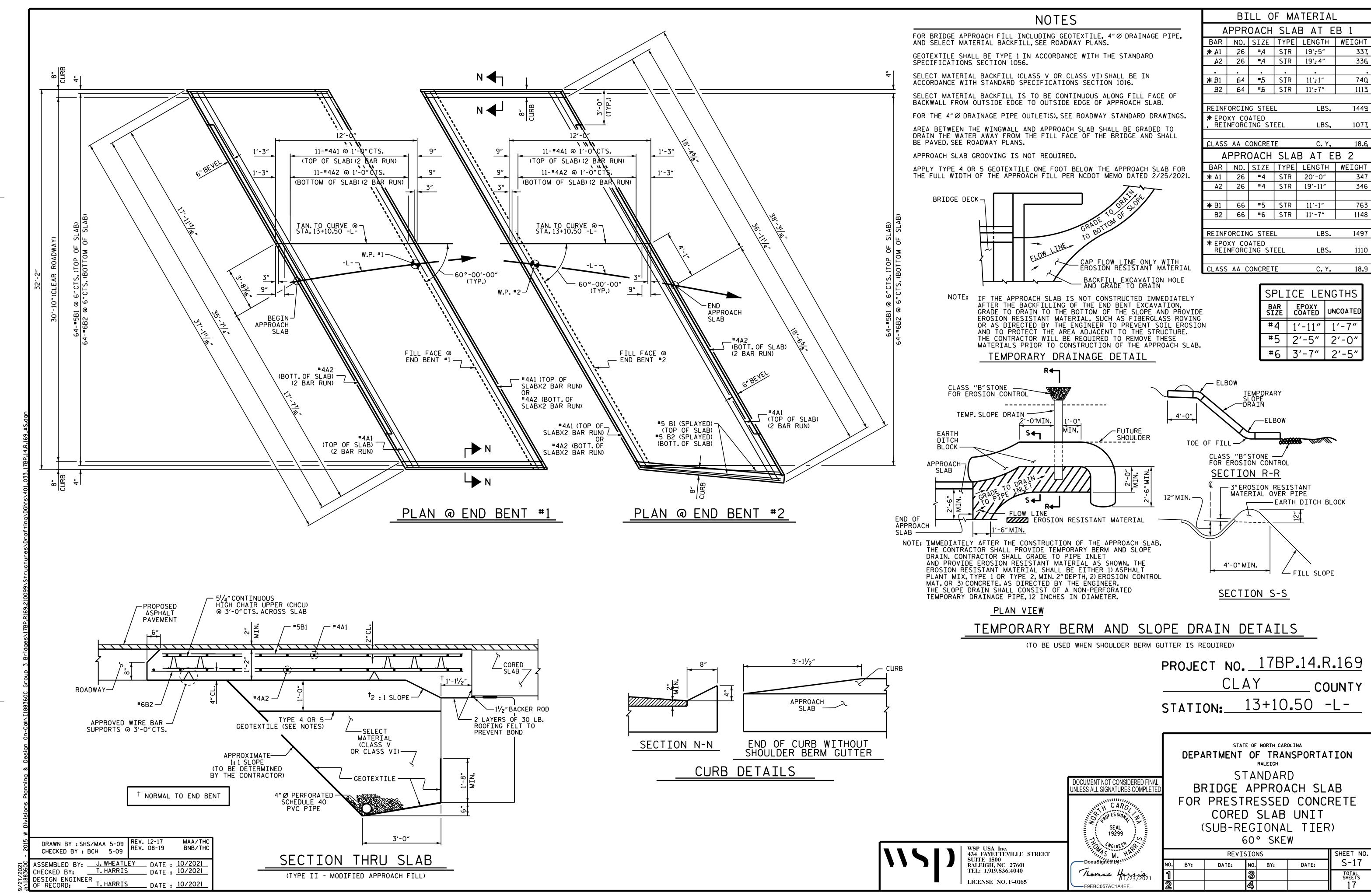
RIP RAP DETAILS

WSP USA Inc.
434 FAYETTEVILLE STREET
SUITE 1500
RALEIGH, NC 27601
TEL: 1.919.836.4040
LICENSE NO. F-0165

SEAL 19300		
19299 SMCINEER HARRING DocuSignéed by 1		
DocuSigned by!	NO.	
Thomas Harris	<u>~</u>	
/homas /arris	Ш	
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DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

REVISIONS						SHEET NO.
	BY:	DATE:	NO.	BY:	DATE:	S-16
			3			TOTAL SHEETS
			4			17



STANDARD NOTES

DESIGN DATA:

---- A.A.S.H.T.O. (CURRENT) IMPACT ALLOWANCE - - - - - - - - - SEE A.A.S.H.T.O. STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - - 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - - 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 - - 27,000 LBS.PER SQ.IN. REINFORCING STEEL IN TENSION - GRADE 60 - - - 24,000 LBS. PER SQ. IN. STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS - - - 1,800 LBS. PER SQ. IN. COMPRESSION PERPENDICULAR TO GRAIN ---- 375 LBS. PER SQ. IN. OF TIMBER EQUIVALENT FLUID PRESSURE OF EARTH - - - - - 30 LBS. PER CU. FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT,

ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{1}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{1}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{1}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{1}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST \(\frac{5}{6}'' \) IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

JANUARY, 1990